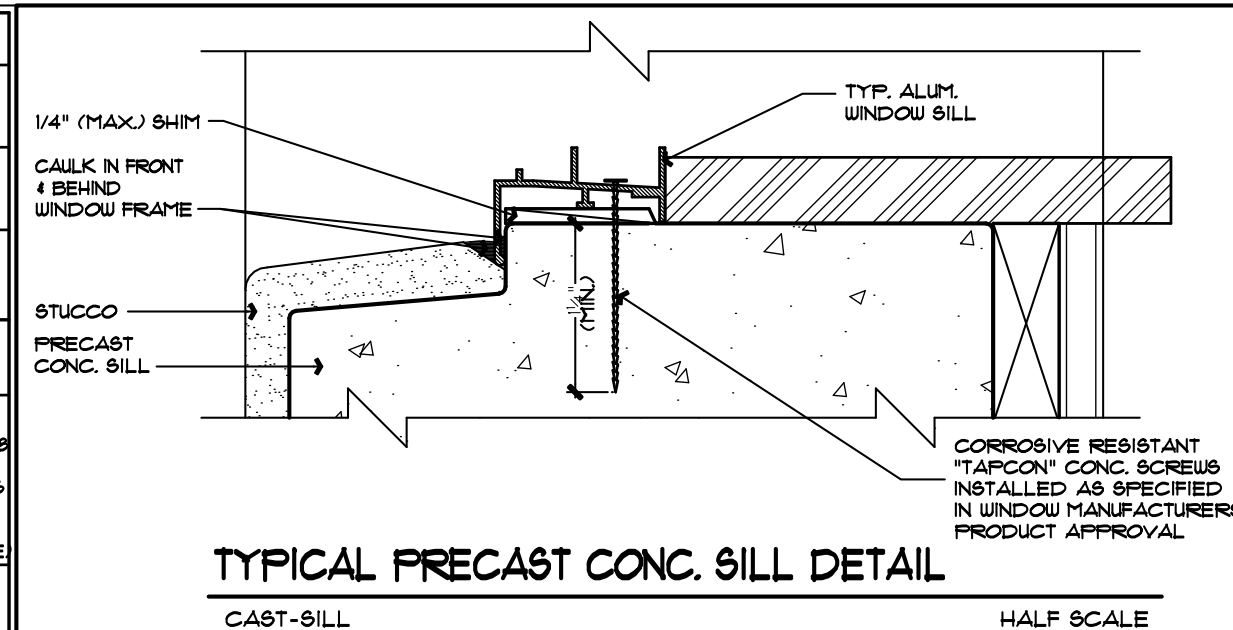


ANCHORAGE SCHEDULE

CONNECTOR	FLORIDA APPROVAL #	MODEL	NO.	FASTENERS TO WOOD	TO CONC.	MAXIMUM ALLOWABLE LOADS	
TRUSS TO CONCRETE	(B) 1325*	FL14T3.9	SIMPSON META D W/ 756 MOISTURE BARRIER	(2) STRAPS (1) MOISTURE BARRIER	(5) 100K(1) 1/2" NAILS EACH STRAP	EMBEDDED LATERAL LOAD PARALLEL TO BM = 3400 (R) 1250 (R)	
	(C) 2500*	FL14T3.3	SIMPSON META W/ 756 MOISTURE BARRIER	(2) STRAPS	(6) 16d NAILS EACH STRAP	EMBEDDED LATERAL LOAD PARALLEL TO BM = 2250 (R) 1250 (R)	
	(F) 120*	FL10456.1	SIMPSON FT5 16	(1)	(6) 16d NAILS	(6) 16d NAILS TO TOP FL.	EMBEDDED LATERAL LOAD PARALLEL TO BM = 2250 (R) 1250 (R)
GABLE TRUSSES	(K) 1025*	FL10655.1	SIMPSON HU42C-2	(1)	(6) 16d NAILS	(8) 1/4"x2 3/4" TITEN HD SCREWS	GRAVITY LOAD = 2400*
	(L) 1810*	FL10655.5	SIMPSON HU42C-2	(1)	(10) 16d NAILS	(14) 1/4"x2 3/4" TITEN HD SCREWS	GRAVITY LOAD = 4350*
	(N) 1020*	FL10655	SIMPSON TR55	(1)	(6) 16d NAILS INTO LEDGER	(6) 16d SLANT NAILS INTO TRUSS	GRAVITY LOAD = 1145*

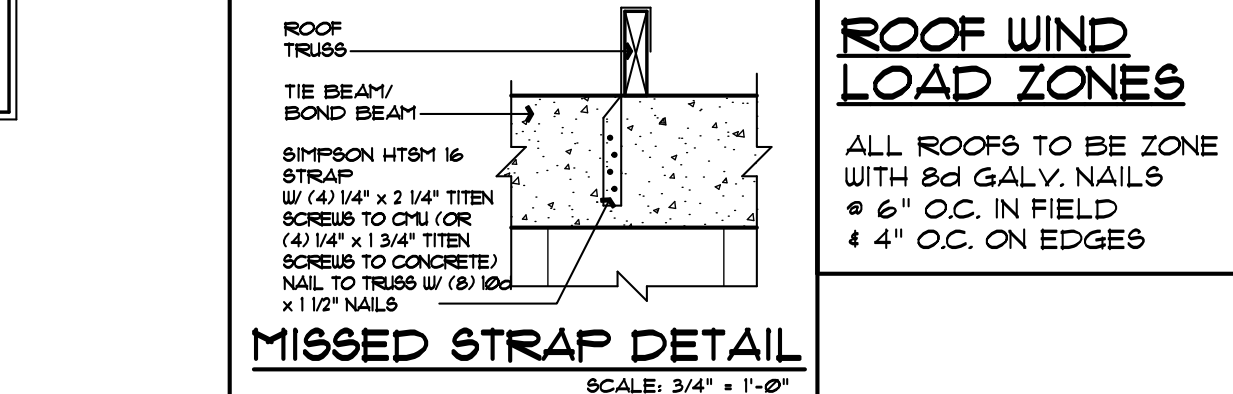
NOTE: ALL MEMBERS TO BE ANCHORED TO CONCRETE BEAM W/ 2" ANCHOR UNLESS OTHERWISE NOTED ON FRAMING PLAN FOR SINGLE 'C' CONNECTIONS ONLY. TO INCREASE A ONE-MEMBER TRUSS TO TWO-MEMBERS, NAIL A 1" LONG 2" STP 2x6 BLOCK OUT TO THE SHAPE OF THE TRUSS AT THE JOINT. TO INCREASE A ONE-MEMBER TRUSS TO TWO MEMBERS, USE A 24" LONG 2" STP 2x6 BLOCK ON THE OPPOSITE FACE OF THE TRUSS NAILED TO THE TRUSS WITH (16) 30" NAILS. ON 'E' & 'F' CONNECTIONS, FOUR ALL CELLS SOLID WITHIN 6" OF TITEN TO SCREWS. DOUBLE STRAPS FOR 'B' & 'C' CONNECTIONS TO BE APPLIED ON ALTERNATING FACES OF TRUSS (1 STRAP ON EA FACE).

GENERAL ANCHOR SCHEDULE NOTES:
 • ALL PRODUCTS TO BE INSTALLED PER MANUFACTURER'S SPECIFICATIONS.
 • STRAP LENGTHS CALLED OUT ABOVE ARE MINIMUM.
 • LIFT LATERAL LOADS AT TRUSS TO WALL CONNECTION ARE TAKEN INTO ACCOUNT IN THE ANCHORAGE SCHEDULE ABOVE.
 • THE UPLIFTS FOR INDIVIDUAL TRUSSES ARE LESS THAN THE VALUES LISTED ABOVE FOR THE CONNECTOR USED TO SECURE THE TRUSS.
 • ALL ANCHORS & RELATIVE NAILS SHALL BE GALVANIZED AS PER FBCC 23316.



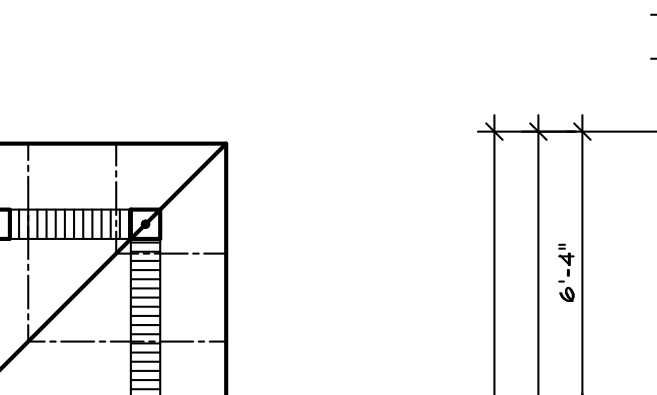
BOND-BEAM SCHEDULE

MARK	SIZE	REINFORCING			#3 HOOPS OR STIRRUPS	REMARKS
		TOP	MID	BTM		
BB-1	8x8	-	-	1#5	NONE REQ.	CONTINUOUS THRU-OUT (SEE TYPICAL WALL SECTION)
L-1	8x8	-	-	-	NONE REQ.	TYPICAL @ ALL OPENINGS GROUT SOLID WITH B.B. ADV. (NO STM. STL. REQ. IN LITE)
L-2	8x8	-	-	1#5	NONE REQ.	PRECAST CONC. LITEL W/ (1) #5 BOT. POURED SOLID W/ B.B. ADV.



ROOF WIND LOAD ZONES

ALL ROOFS TO BE ZONE 3 WITH 8d GALV. NAILS @ 6" O.C. IN FIELD & 4" O.C. ON EDGES

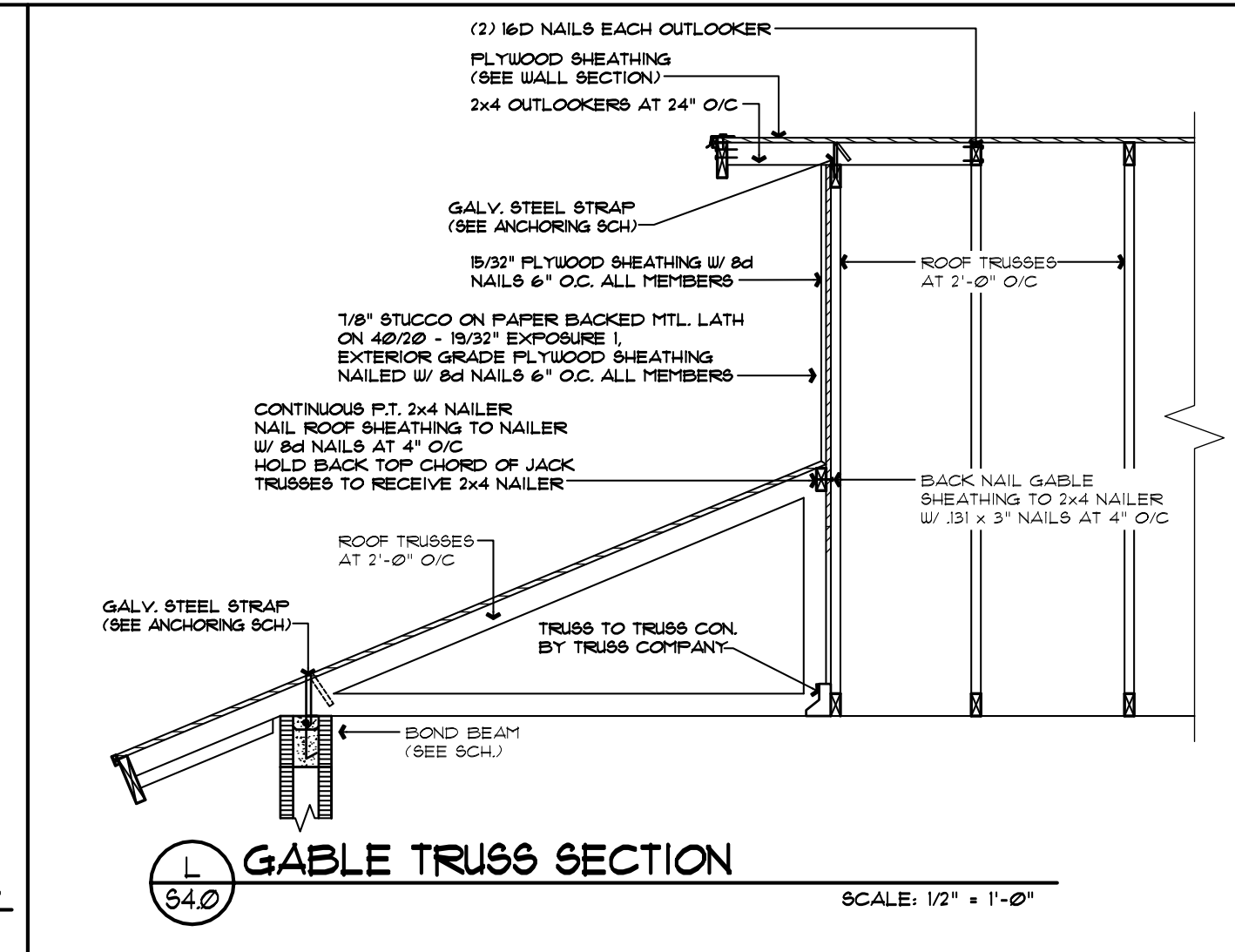
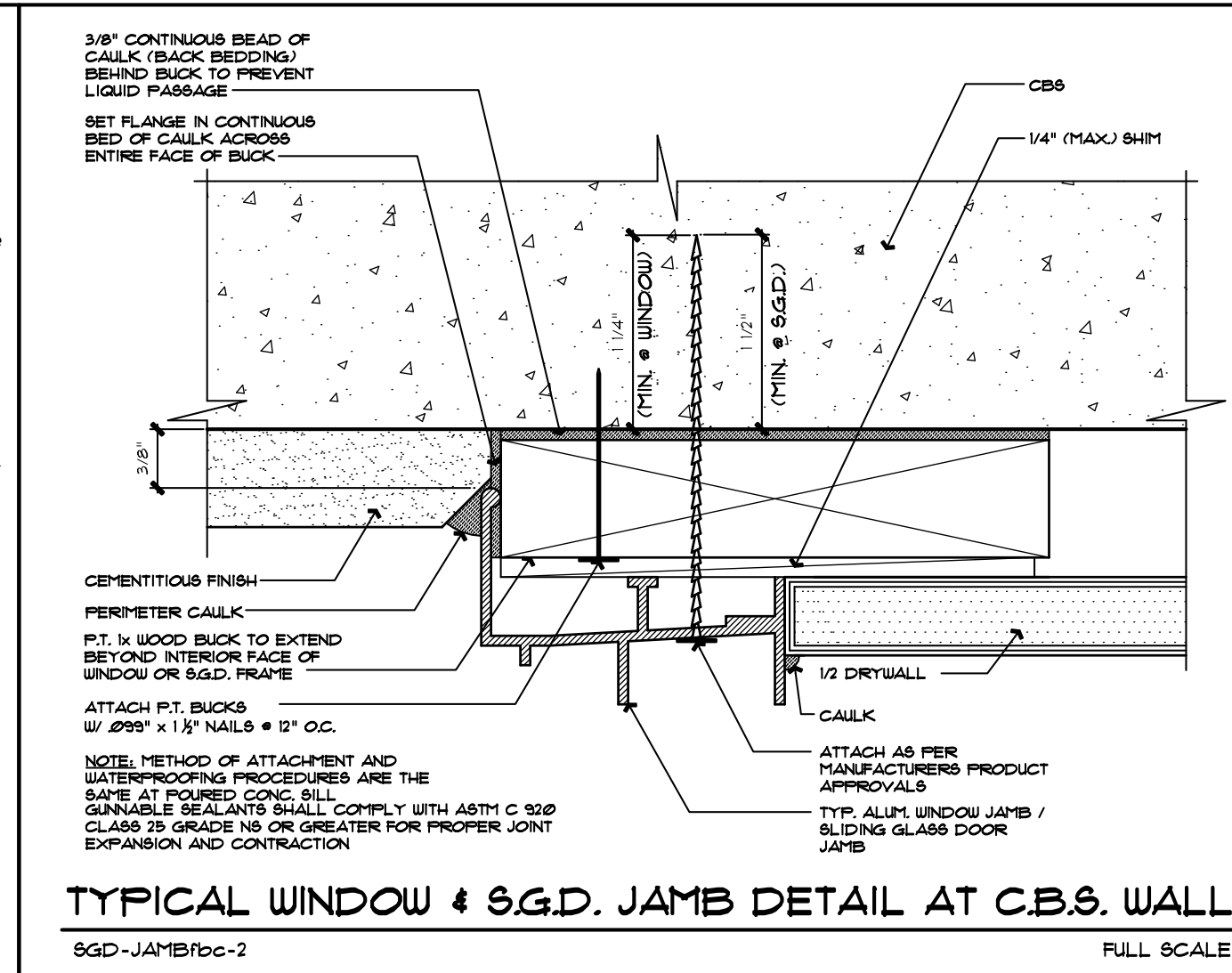


SCALE: 3/4" = 1'-0"

FOUNDATION NOTES

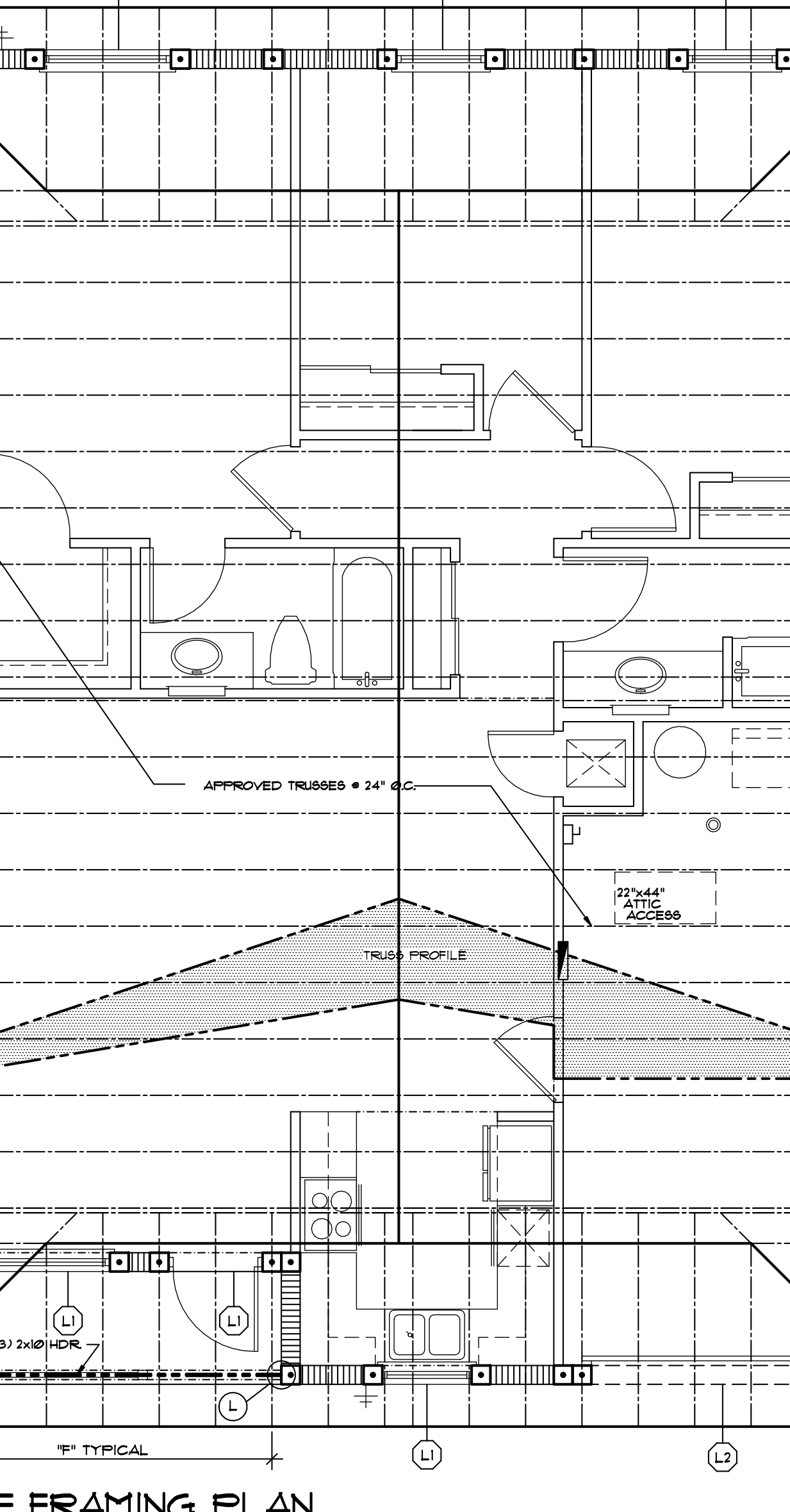
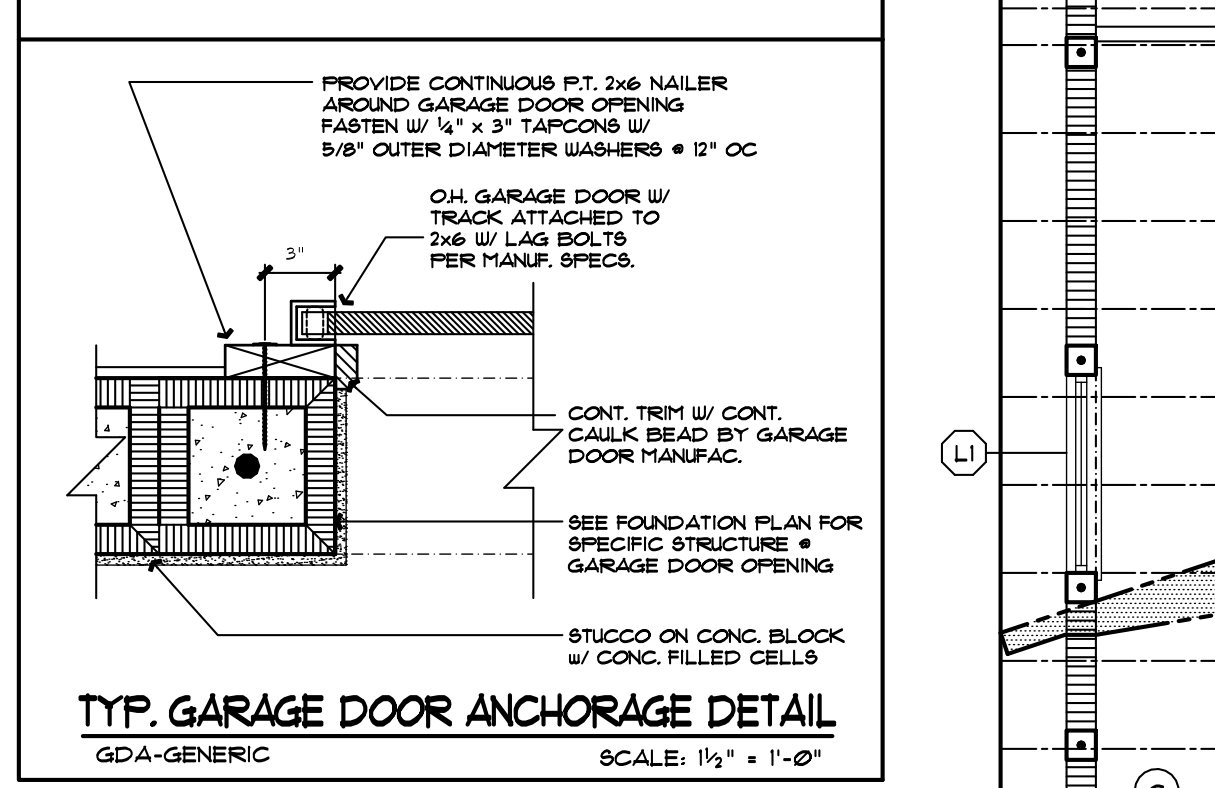
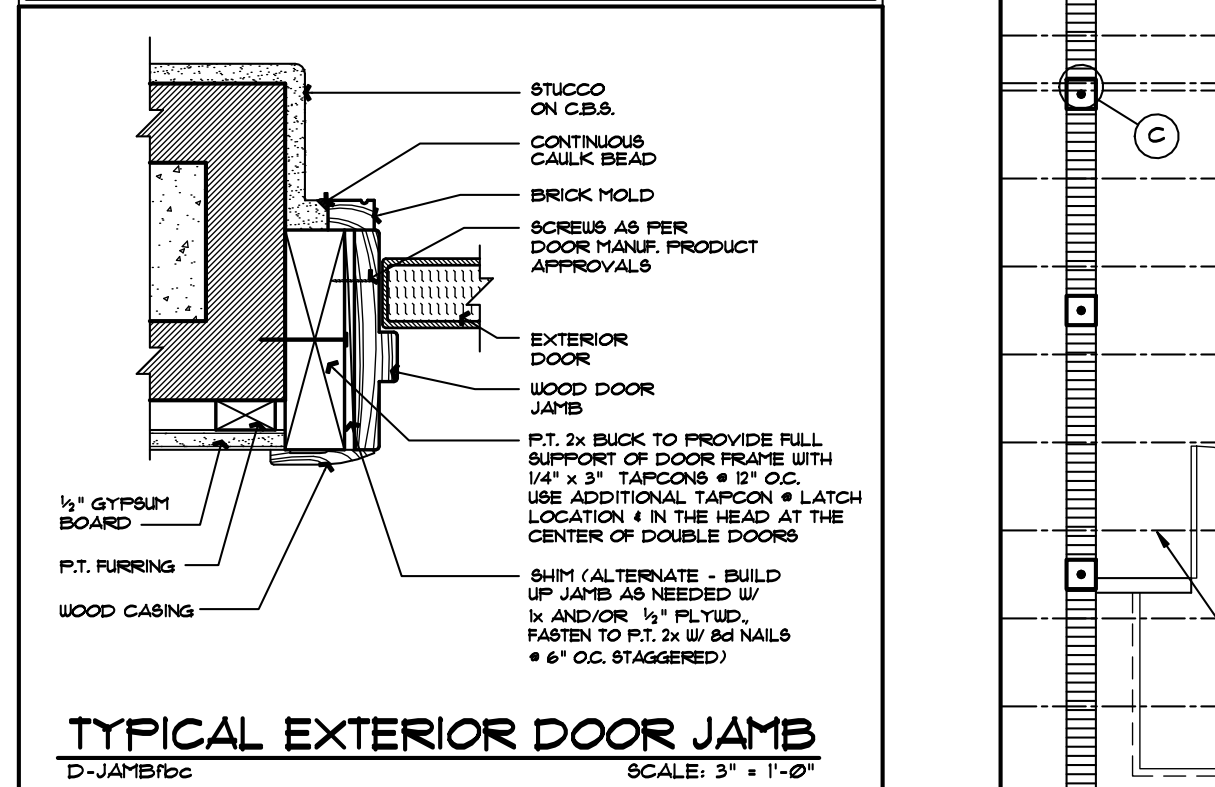
- Floor slab to be 4" thick poured concrete slab with 6" x 6" x 10/10 W.W.M. on 0.002 slope (or equal) on clean well compacted fill pre treated for termites. (Fiber-mesh concrete additive may be substituted for 6" x 6" x 10/10 woven wire mesh)
- All reinforcing steel to be grade 60. Conc. strength shall be 2500 P.S.I. at 28 days
- Column and Wall Centerlines shall coincide with footing centerlines unless otherwise noted.
- Footing design based on assumed soil bearing capacity of 2500 P.S.F., contractor to verify.
- Fill placed within 5'-0" of the construction perimeter shall consist of clean well graded sand in 12" lifts (max.) and vibratory compacted to achieve a minimum of 95% modified proctor ASH10-F180.
- ALL FOUNDATION REINFORCEMENT STEEL shall be cont. @ all corners - use corner bars w/ 30" lap splice min.

GENERAL NOTES: Monolithic footing preparation
 1. After standard cleaning and grubbing has been completed and approved, apply vibratory compactor with a minimum of four passes to the existing ground.
 2. Check the density of existing ground to insure minimum density of 95% of modified proctor has been reached from top of existing ground to depth of 2'-0" below grade.
 3. After existing ground reaches minimum density of 95%, fill shall be placed in 8" lifts and compacted to achieve 95% modified proctor. Fill shall be clean free from deleterious materials.
 4. Verification of test for modified proctor ASH10-F180 shall be filed with building officials.

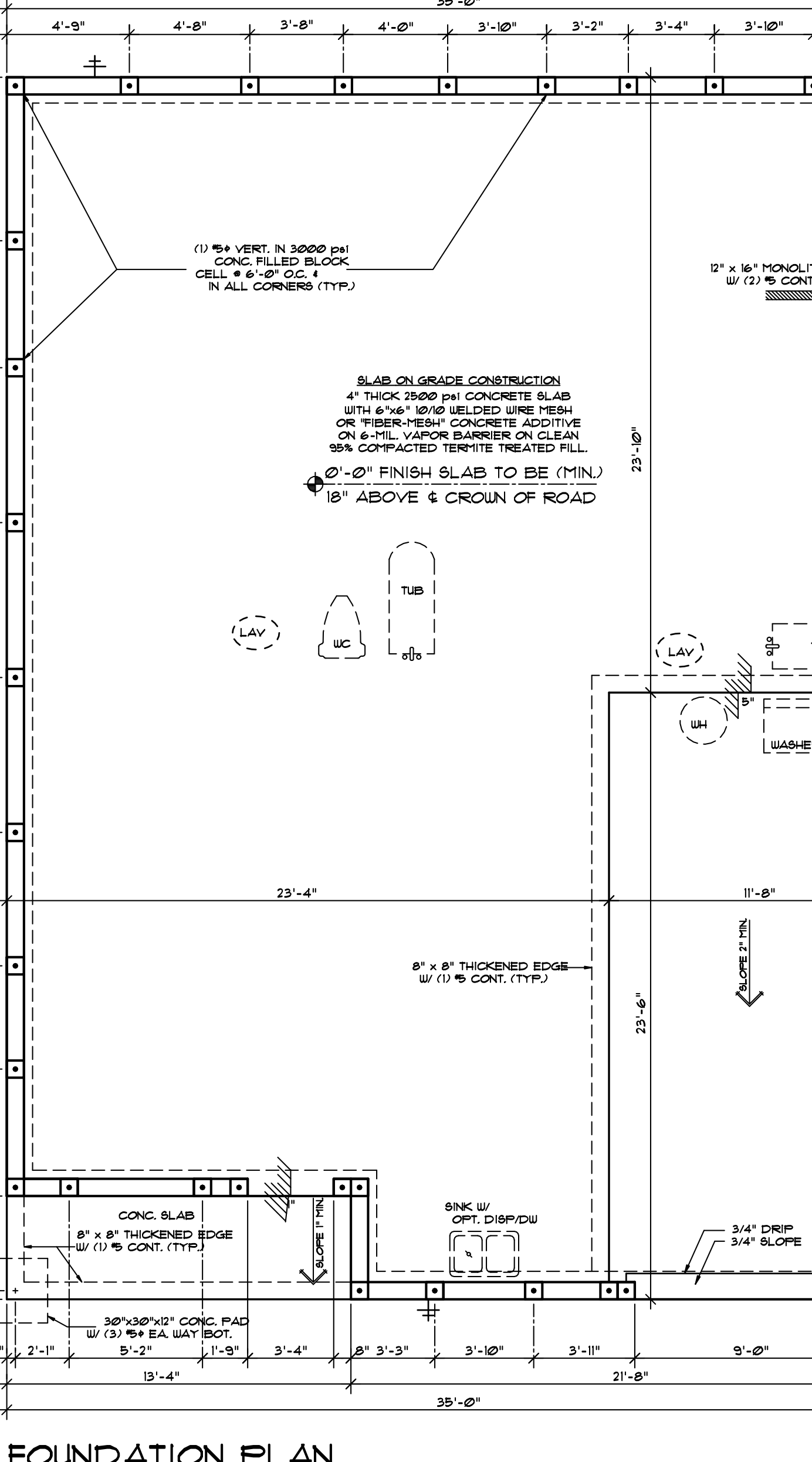


FRAMING PLAN NOTES

- THE ROOF FRAMING PLAN SHOWS LOCATION & DIRECTION OF ALL ROOF MEMBERS (TRUSSES AND CHORDS) FOR INDIVIDUAL TRUSSES, SEE LAYOUT & ENGINEERING SHEETS BY TRUSS MANUFACTURER.
- ALL CONCRETE BEAMS TO BE A MINIMUM OF 8" x 12" WITH (2) #5 BARS TOP & (2) #5 BARS BOTTOM UNLESS OTHERWISE NOTED ON PLANS.
- AT ALL POURED CONCRETE BEAM AND BOND BEAM INTERSECTIONS, EXTEND BOND BEAM STEEL 36" INTO POURED BEAMS.
- ANY OF THE SPECIFICALLY NOTED BEAMS MAY BE BUILT UP TO 1" SMALLER OR 2" LARGER WITHOUT ANY CHANGE OF REQUIRED STEEL.
- STIRRUPS NOT REQUIRED UNLESS NOTED OTHERWISE.
- ALL HEADERS & WOOD BEAMS TO BE (2) 2"x12" HEM-FIR #2 UNLESS NOTED OTHERWISE ON PLAN.
- 2" N.S.P. MAY BE SUBSTITUTED ANYWHERE HEM-FIR IS SPECIFIED.
- PROVIDE FILLED CELLS - MINIMUM 6" ABOVE & 6" BELOW ALL EXPANSION BOLT LOCATIONS IN C.B.S. WALLS (TYPICAL).
- SPACE FLOOR TRUSSES ACCORDINGLY TO AVOID CONFLICT W/ PLUMBING FIXTURE DRAINS ABOVE. COORDINATE W/ ARCHITECTURAL PLANS.



ROOF FRAMING PLAN
SCALE: 1/4" = 1'-0"



FOUNDATION PLAN
SCALE: 1/4" = 1'-0"

STRUCTURAL DESIGN STATEMENT

I CERTIFY THAT, TO THE BEST OF MY KNOWLEDGE AND BELIEF, THESE PLANS AND SPECIFICATIONS HAVE BEEN DESIGNED TO COMPLY WITH THE APPLICABLE STRUCTURAL PORTIONS OF THE BUILDING CODES.

I ALSO CERTIFY THAT THE STRUCTURAL COMPONENTS, SYSTEMS, & RELATED ELEMENTS PROVIDE ADEQUATE RESISTANCE TO WIND LOADS AND FORCES SPECIFIED BY THE CODES LISTED BELOW.

DESIGN PARAMETERS & ASSUMPTIONS

CODE EDITION:
 FLORIDA BUILDING CODE 6th EDITION (2017)
 ASCE 1-10 (EXPOSURE CONDITION, INDICATE C)
 CONSTRUCTION TYPE: V, E
 OCCUPANCY CLASSIFICATION: R-3

BUILDING DESIGNED AS:
 PARTIALLY ENCLOSED ENCLOSED OPEN TESTED (AND TUNNEL)

BUILDING HEIGHT:
 ≤ 60ft. (ASCE 1-10)
 > 60ft. (MUST USE ASCE 1)

MEAN ROOF HEIGHT: 15'-0" MAX.
 (FOR LOW RISE BLDG. WITH SLOPED ROOFS)

RISK FACTOR II:
 (DETERMINED BY BUILDING USE, OCCUPANCY, REFER TO ASCE 1-10 TABLE 15-1)

INTERNAL PRESSURE COEFFICIENT: ±1.0
 (ENCLOSED BUILDING)

BASIC WIND VELOCITY PRESSURES:
 APPROPRIATE POSITIVE / NEGATIVE PRESSURE COEFFICIENTS HAVE BEEN APPLIED TO MAIN WIND FORCE RESISTING SYSTEM, AND BUILDING ENVELOPE COMPONENTS AND GLAZINGS, AS APPLICABLE.

WIND SPEED: 110 (3-SEC. GUST) BASIC VELOCITY PRESSURE 32.07 PSF
 NOTE: ACTUAL DESIGN PRESSURES FOR ALL EXTERIOR WINDOWS, DOORS, GARAGE DOORS, AND SIMILAR ENVELOPE ELEMENTS ARE INDICATED ON CONSTRUCTION PLANS.

ROOF TRUSS TOP CHORD LIVE LOAD: 20 PSF
ROOF TRUSS BOTTOM CHORD LIVE LOAD: 10 PSF
 2000 LB. CONCENTRATED LOAD AT ALL BOTTOM CHORD PANEL JOINTS (HVHZ ONLY) - NON-CONCURRENT

ROOF TRUSS DEAD LOAD: 15 PSF FOR TOP CHORD
 10 PSF FOR BOTTOM CHORD
ROOF TRUSS DEAD LOAD TO RESIST WIND UPLIFT: 6 PSF TOTAL

SOIL BEARING CAPACITY: 2500 PSF
 REVIEWED FOR SHEAR WALL REQUIREMENTS: YES NO
 IMPACT PROTECTION REQUIRED: YES NO

110 N.W. 6TH AVE. BOYNTON BEACH
SINGLE FAMILY RESIDENCE for:
HABITAT for HUMANITY of S.P.B.C.
FOUND. & FRAMING PLANS

plan #: 1329-GR	WALSH ENGINEERING, Inc. 3200 N. Federal Hwy., Suite 226 Boca Raton, FL 33431	sheet #: 1
job #: 19B594	(561) 362-0237 PE 0000 EB 0000	
date: 3/28/19	McCARRON / ASSOC., Inc. AIBD/FRDA - 638 EDDY ST., BOCA RATON, FL 33487 561-997-0579 /- 561-350-4364 mccarronassoc@aif.net - www.mccarroninc.com	3